

EXHIBIT B - ENGINEERED WALL PLAN



SEC

Symons Engineering Consultants, Inc.

12805 S.E. Foster Road

Portland, OR 97236

(503) 760-1353

Fax 762-1962

MEMO

TO: Ron Bistline

DATE: September 19, 2018

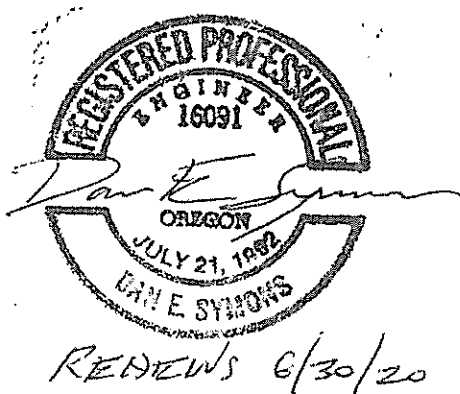
FIRM: 716 4th Street
Oregon City, OR

FROM: Dan Symons

PROJECT: 2' Gravity Wall in ROW

PROJECT No: 15-08

At your request I have reviewed a 2' high rockery wall proposed in 4th Street in front of the above address. If the wall is constructed in accordance with and, at a minimum, to meet or exceed the attached requirements then the installation shall be considered in general conformance with industry standards.



Symons Engineering Consultants, Inc.			GRAVITY		Project: Bistline	
12805 SE Foster Rd			Gravity Boulder		Project #: 15-08	
Portland, OR 97236			GW= 2 FT		Date: 9/19/18	
503-760-1353, fax: 762-1962						

Retaining Wall Properties:			
W=	1.5	ft	Wall Base Width
Hw	2	ft	Wall Ht
H1=	3	ft	Grade Slope Soil Ht
Ht=	0.5	ft	Toe Depth Below Grade
Pb=	1500	psf	Allowable Soil Bearing
Pp=	300	Psf/ft	Passive Soil Pressure
f=	0.6		Soil Friction Coeff
Ws=	120	pcf	Soil Unit Wt
Wr=	145	pcf	Rock Unit Wt
Br=	6		Wall Batter of Face
Sp=	2		Grade Slope

Active Soil Pressure from Appendix A Chart:			
β =	26.6	deg	Slope Angle
H=	2.5	ft	Retained Wall Ht
H1/H=	1.20		Ht Ratio
Kh=	60	psf/ft	Horizontal Active Soil Pressure
Kv=	27	psf/ft	Vertical Active Soil Pressure

Active Soil Pressure Recation:			
Rh=	0.19	kips	Horizontal Force
Rv=	0.08	kips	Vertical Force
		$Rh = (1/2) * Kh * H^2$ $Rv = (1/2) * Kv * H^2$	

Overturning Moment about Heel of Wall:			
H/3=	0.83	ft	Moment Arm
OM=	0.16	k-ft	Overturning Moment
		$OM = Rh * (H/3)$	

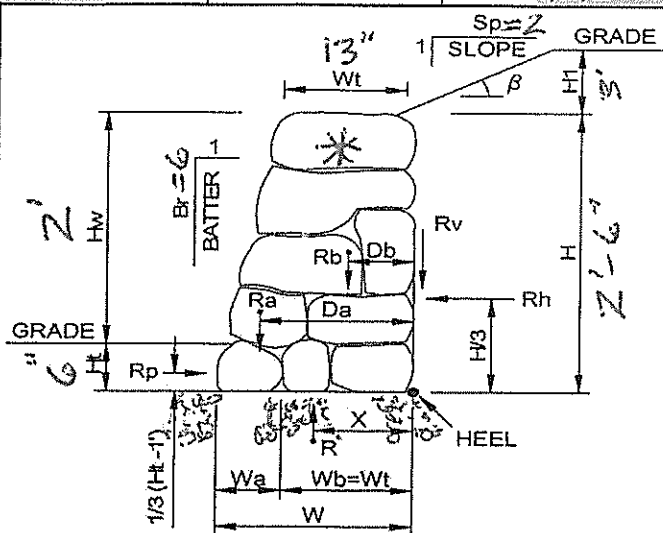
Vertical Reaction Force From Boulder Rock Wt:			
Wt=	1.08	ft	Top width of Wall
Wa=	0.42	ft	Wall Batter Base Width
Wb=	1.08	ft	Wall Base Width
Ra=	0.08	kips	Batter Vertical Reaction
Rb=	0.39	kips	Base Vertical Reaction
R=	0.47	kips	Vertical Resultant
		$Wt = Wb$ $Wa = H/Br$ $Wb = W - Wa$ $Ra = (1/2) * Wa * H * Wr$ $Rb = Wb * H * Wr$ $R = Ra + Rb$	

Resistive Moment about Heel of Wall:			
Da=	1.22	ft	Moment Arm
Db=	0.54	ft	Moment Arm
X=	0.99	OK	
RM=	0.46	k-ft	Resistive Moment
		$Da = (1/3) * Wa + Wb$ $Db = (1/2) * Wb$ $X = (OM + Ra * Da + Rb * Db) / R$ $RM = R * X$	

Overturning Factor of Safety:			
OM F.S.=	2.95	OK	Overturning Factor of Safety
		$OM F.S. = RM / OM \geq 1.5$	

Sliding Factor of Safety:			
Rp=	0.00	kips	Passive Soil Pressure at Toe
P=	0.47	kips	Vertical Wall Wt
f*P=	0.28	kips	Sliding friction
Rh=	0.19	kips	Horizontal Force
Sliding F. S.=	1.50	N.G.	Sliding Factor of Safety
		$Rp = (1/2) * Pp * (Ht-1)^2$ $P = Ra + Rb + Rv$ $Rh = (1/2) * Kh * H^2$ $Sliding F.S. = (f * P + Rp) / Rh \geq 1.5$	

Soil Bearing Pressure:			
P=	0.47	kips	Vertical Wall Wt
A=	1.50	sf	Base Area per Foot Width
M=	-0.11	k-ft	Moment
c=	0.75	ft	Moment Arm
I=	0.28	ft^4	Moment of Inertia
Pt=	606	psf	Toe Bearing Pressure
Pt=	19	psf	Heel Bearing Pressure
Toe Unity=	0.40	OK	Toe Bearing Pressure Unity
Heel Unity=	0.01	OK	Heel Bearing Pressure Unity
		$M = (1/3) * (Ht-1) * Rp + (1/2) * W * Rv - (1/3) * H * Rh + Rb(W/2 - Db) - Ra(Da - W/2)$ $c = (1/2) * W$ $I = (1/12) * W^4$ $Ph = P/A - M * c / I$ $Pt = P/A + M * c / I$ $Toe Unity = Pt / Pb \leq 1.0$ $Heel Unity = Ph / Pb \leq 1.0$	



18"

24" x 6"

THICK 3/4(-)

CRUSHED

ROCK BASE

ANGULAR

* STONES SHALL BE

SELECTIVELY

HAND PLACED TO

ENSURE FULL

INTERLOCK

NO INTERNAL

SLIPPAGE &

MINIMAL VOIDS

