Pavilion III

25- Lot proposed development

19588 S McCord Road

Oregon City, Oregon



# PRELIMINARY DRAINAGE REPORT April 2015

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# 2014-129H



9/4/15

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# NARRATIVE ASSUMPTIONS

# Existing Conditions:

The subject property has a large house with outbuilding and driveway to McCord. The majority of the site is a Christmas tree farm generally sloping from east to west. There are two subbasins on the property but both would result in flow to McCord Road and northerly along the Road to a natural drainage way approximately 250-feet northerly of the property. Pavilion Park I abuts the project to the north with road connections and there is a detention/water quality facility approximately 200-feet north on McCord Road.

# Developed conditions:

25-new lots are proposed which will extend Anita Place, Pelican Lake Place and Villard Place and complete Joseph Way. McCord Road would be improved with a half street. Storm water would be directed to the original Pavilion system with the majority flowing in a new line north on McCord Road and into the existing detentions facility. This facility would be enlarged to receive and process the new development.

# **Drain Basin Description:**

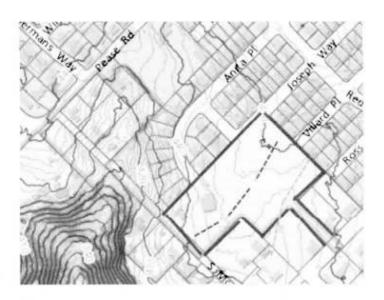
# Existing

There are two minor basins on the property which divide the property more or less in half. The discharge point for both is the roadside ditch on the easterly side of McCord Road. The

northerly basin flows directly to McCord road while the southerly basin flows south westerly across other properties before reaching McCord road. Once the flow reaches the roadside ditch the basins are combined and enter the natural drainage way northwesterly of the site.

# Developed

In the developed condition the two basins will be combined with storm lines running westerly in Villard Place, thence in McCord to the existing detention/water quality facility. Overall the drainage pattern will be the same. No discharge of impervious storm water will flow onto adjacent properties. The enlarged facility will meter the storm water out at the pre-design rates.



# Summary of storm water flow

	2-YEAR	5-YEAR	10-YEAR	25-YEAR
PRE-DEVELOP	1.62CFS	2.07CFS	2.35CFS	2.89CFS
POST-DEVELOP	2.17CFS	2.74CFS	3.08CFS	3.78 CFS

DESIGN STORM	REQUIRED RELEASE	DESIGN RELEASE
25 YEAR-24 HR	2.35 CFS	2.34CFS
10 YEAR -24 HR	N/A	N/A
5 YEAR - 24 HR	2.07 CFS	0.92CFS
2 YEAR -24 HR	0.81 CFS	0.81 CFS

# REGULATORY DESIGN CRITERIA

The storm water quantity management requirements of Oregon City are:

- City Code 13.12 Storm water management and the 1988 Drainage Mater Plan
- City of Oregon City, Public Works, Storm Water & Grading Design Standards.

# References

 King County Department of Public Works, Surface Water Management Division, Hydrographic Programs, Version 4.21B

# **Water Quality Facility**

The required treatment rate is 1/3 of the 2-year design storm. For this project the calculations the 2-year storm is 1.37CF and the water quality quantity is 0.46CFS. The following options will be considered in the final design: a swale inside the detention pond, infiltration at the bottom of the facility or a Stormceptor by CRS with the capacity to treat 100% of the flows.

# **Design Parameters**

The design storm is a 24 hour standard SCS Type 1A

•	2-year	2.6	inches
•	5-year	3.1	inches
•	25-year	4.0	inches
			inches

#### SOIL TYPES

8B Bornstedt silt - Type C soil

24B Cottrell Silty/clay - Type C soil

# Time of Concentration

$$T = 0.42(n L)^{.8}/(P_2)^{0.5}(S_0)^{0.4} & T = L/60k(s_0)^{0.5}$$

Pre-Development:  $(.42)[(0.17(300)]^{0.8}/(2.6)^{0.5}(0.014)^4 = 33.4 \text{ min } \& 350/(60)(11)(0.023)^{.5} = 3.5 \text{min} : total 36.9 \text{ minutes}$ 

Post-Development  $(.42)[(0.15(133)]^{0.8}/(2.6)^{0.5}(0.020)^4 = 13.7 \text{ min } 233/(60)(27)(0.010)^{.5} = 1.4 \text{ min } + 270/(60)(42)(0.02)^{.5} = 0.7 \text{ min: Total } 15.8 \text{ minutes}$ 

# HYDROGRAPH RESULTS

KING COUNTY DEPARTMENT OF PUBLIC WORKS

# Surface Water Management Division

# HYDROGRAPH PROGRAMS

Version 4.21B

- 1 INFO ON THIS PROGRAM
- 2 SBUHYD
- 3 MODIFIELD SBUHYD
- 4 ROUTE
- 5 ROUTE2
- 6 ADDHYD
- 7 BASEFLOW
- 8 PLOTHYD
- 9 DTATA
- 10 REFAC
- 11 RETURN TO DOS

ENTER OPTION:

7

SBUN/SCS METHOD FOR COMPUTING RUNOFF HYDROGRAPH

STORM OPTIONS:

- 1 S.C.S. TYPE-1A
- 2 7-DAY DESIGN STORM
- 3 STORM DATA FILE

SPECIFY STORM OPTION:

1

S.C.S. TYPE - 1A RAINFALL DISTRIBUTION

ENTER; FREQ(YEAR), DURATION(HOUR), PRECIP(INCHES)

2,24,2.6

ENTER: A(PERV), CN(PERV), A(IMPERV), CN(IMPERV), TC FOR BASIN NO. 1

5.08,92,0.18,98,36.9

DATA PRINT OUT:

AREA(ACRES)	PERV	ious	IMPE	RVIOUS	TC(MINUTES)
	Α	CN	Α	CN	
5.3	5.1	92	.2	98	36.9
PEAK-Q(CFS)	T-PEA	K(HRS)	VOL(	CU-FT)	
1.62	7.	83	34	4181	

ENTER [d:][path]filename[.ext] FOR STORAGE OF COMPUTED HYDROGRAPH:

C:piii2pre

SPECIFY: C - CONTINUE, N - NEWSTORM, P - PRINT, S - STOP

C

ENTER: A(PERV), CN(PERV), A(IMPERV), CN(IMPERV), TC FOR BASIN NO. 1

2.68,86,2.58,98,15.8

DATA PRINT OUT:

AREA(ACRES)	PERVI	ous	IMPE	RVIOUS	TC(MINUTES)
	Α	CN	Α	CN	
5.3	2.7	86	2.6	98	15.8
PEAK-Q(CFS)	T-PEA	K(HRS)	VOL(C	CU-FT)	
2.17	7.8	33	350	016	

ENTER [d:][path]filename[.ext] FOR STORAGE OF COMPUTED HYDROGRAPH:

C:piii2post

SPECIFY: C - CONTINUE, N - NEWSTORM, P - PRINT, S - STOP

STORM OPTIONS:

- 1 S.C.S. TYPE-1A
- 2 7-DAY DESIGN STORM
- 3 STORM DATA FILE

SPECIFY STORM OPTION:

1

S.C.S. TYPE - 1A RAINFALL DISTRIBUTION

ENTER; FREQ(YEAR), DURATION(HOUR), PRECIP(INCHES)

5,24,3.1

ENTER: A(PERV), CN(PERV), A(IMPERV), CN(IMPERV), TC FOR BASIN NO. 1

5.08,92,0.18,98,36.9

DATA PRINT OUT:

AREA(ACRES)	PERVI	ous	IMPE	RVIOUS	TC(MINUTES)
	Α	CN	Α	CN	
5.3	5.1	92	.2	98	36.9
PEAK-Q(CFS)	T-PEA	K(HRS)	VOL(	CU-FT)	
2.07	7.8	3	43:	105	

ENTER [d:][path]filename[.ext] FOR STORAGE OF COMPUTED HYDROGRAPH:

C:pili5pre

SPECIFY: C-CONTINUE, N-NEWSTORM, P-PRINT, S-STOP

C

ENTER: A(PERV), CN(PERV), A(IMPERV), CN(IMPERV), TC FOR BASIN NO. 1

2.68,86,2.58,98,15.8

DATA PRINT OUT:

AREA(ACRES) PERVIOUS IMPERVIOUS TC(MINUTES)

A CN A CN

5.3 2.7 86 2.6 98 15.8

PEAK-Q(CFS) T-PEAK(HRS) VOL(CU-FT)

2.74 7.83 43775

ENTER [d:][path]filename[.ext] FOR STORAGE OF COMPUTED HYDROGRAPH:

C:piii5post

SPECIFY: C-CONTINUE, N-NEWSTORM, P-PRINT, S-STOP

N

STORM OPTIONS:

1 - s.c.s. TYPE-1A

2 - 7-DAY DESIGN STORM

3 - STORM DATA FILE

SPECIFY STORM OPTION:

1

S.C.S. TYPE - 1A RAINFALL DISTRIBUTION

ENTER; FREQ(YEAR), DURATION(HOUR), PRECIP(INCHES)

10,24,3.4

ENTER: A(PERV), CN(PERV), A(IMPERV), CN(IMPERV), TC FOR BASIN NO. 1

5.08,92,018,98,36.9

DATA PRINT OUT:

IMPERVIOUS TC(MINUTES) AREA(ACRES) PERVIOUS CN CN A 5.1 92 .2 98 36.9 5.3 T-PEAK(HRS) VOL(CU-FT) PEAK-Q(CFS) 7.83 48524 2.35

ENTER [d:][path]filename[.ext] FOR STORAGE OF COMPUTED HYDROGRAPH:

C:piii10pre

C

ENTER: A(PERV), CN(PERV), A(IMPERV), CN(IMPERV), TC FOR BASIN NO. 1

2.68.86,2.58,98,15.8

DATA PRINT OUT:

AREA(ACRES)	PERVI	ous	IMPE	RVIOUS	TC(MINUTES)
	Α	CN	Α	CN	
5.3	2.7	86	2.6	98	15.8
PEAK-Q(CFS)	T-PEA	K(HRS)	VOL(C	CU-FT)	
3.08	7.8	3	491	09	

ENTER [d:][path]filename[.ext] FOR STORAGE OF COMPUTED HYDROGRAPH:

C:piii10post

SPECIFY: C-CONTINUE, N-NEWSTORM, P-PRINT, S-STOP

N

STORM OPTIONS:

- 1 S.C.S., TYPE-1A
- 2 7-DAY DESIGN STORM
- 3 STORM DATA FILE

SPECIFY STORM OPTION:

1

S.C.S. TYPE - 1A RAINFALL DISTRIBUTION

ENTER; FREQ(YEAR), DURATION(HOUR), PRECIP(INCHES)

25,24,4

ENTER: A(PERV), CN(PERV), A(IMPERV), CN(IMPERV), TC FOR BASIN NO. 1

5.08,92,0.18,98,36.9

# DATA PRINT OUT:

AREA(ACRES)	PERVI	ous	IMPE	RVIOUS	TC(MINUTES)
	Α	CN	Α	CN	
5.3	5.1	92	.2	98	36.9
PEAK-Q(CFS)	T-PEA	K(HRS)	VOL(	CU-FT)	
2.89	7.83	Ĺ	59	468	

ENTER [d:][path]filename[.ext] FOR STORAGE OF COMPUTED HYDROGRAPH:

C:piii25pre

C

ENTER: A(PERV), CN(PERV), A(IMPERV), CN(IMPERV), TC FOR BASIN NO. 1

2.68,86,2.58,98,15.8

DATA PRINT OUT:

AREA(ACRES)	PERVI	ous	IMPE	RVIOUS	TC(MINUTES)
	Α	CN	Α	CN	
5.3	2.7	86	2.6	98	15.8
PEAK-Q(CFS)	T-PEA	K(HRS)	VOL(C	CU-FT)	
3.78	7.83	3	59	909	

ENTER [d:][path]filename[.ext] FOR STORAGE OF COMPUTED HYDROGRAPH:

C:br25post

**DETENTION SIZING** 

**ENTER OPTION** 

10

R/D FACILITY DESIGN ROUTINE

SPEFICY TYPE OF R/D FACILTY

1 - POND

4 - INFILTRATION POND

2 - TANK

5 - INFILTRATION TANK

```
3 -VAULT 6 - GRAVEL TRENCH/BED
1
ENTER: POND SIDE SLOPE (HORIZ. COMPOENT)
3
ENTER: EFFECTIVE STORAGE DEPTH(ft) BEFORE OVERFLOW
3
ENTER [d:][path]filename[.ext] OF PRIMARY DESIGN INFLOW HYDROGRAPH:
C:br25post
PRELIMINARY DESIGN INFLOW PEAK = 3.78
ENGER PRIMARY DESIGN RELEASE RATE(cfs)
2.35
ENTER NUMBER OF INFLOW HYDROGRAPHS TO BE TESTED FOR PERFORMANCE (5 MAXIMUM)
2
ENTER [d:][path]filename[ext] OF HYDROGRAPH 1:
C:piii5post
ENTER TARGET RELEASE RATE(cfs)
2.07
ENTER [d:][path]filename[ext] OF HYDROGRAPH 2:
C:piii2post
 ENTER TARGET RELEASE RATE(cfs)
 .0.81
 ENTER: NUMBER OF ORIFICES, RISER-HEAD(ft), RISER-DIAMETER(in)
 2,4,15
 RISER OVERFLOW DEPTH FOR PRIMARY PEAK INFLOW= 0.46FT
 SPECIFY ITERATION DISPLAY: Y -YES, N - NO
```

SPECIFY: R - REVIEW/REVISE INPUT, C - CONTINUE

INITIAL STORAGE VALUE FOR ITERATION PURPOSES: 20040 CU-FT

BOTTOM ORIFICE: ENTER Q-MAX(cfs)

1.0

DIA.=4.61 INCHES

TOP ORIFICE: ENTER HEIGHT (ft)

2.7

DIA.= 9.53 INCHES

PERFORMANCE: INFLOW TARGET-OUTFLOW ACTUAL-OUTFLOW PK-STAGE STORAGE

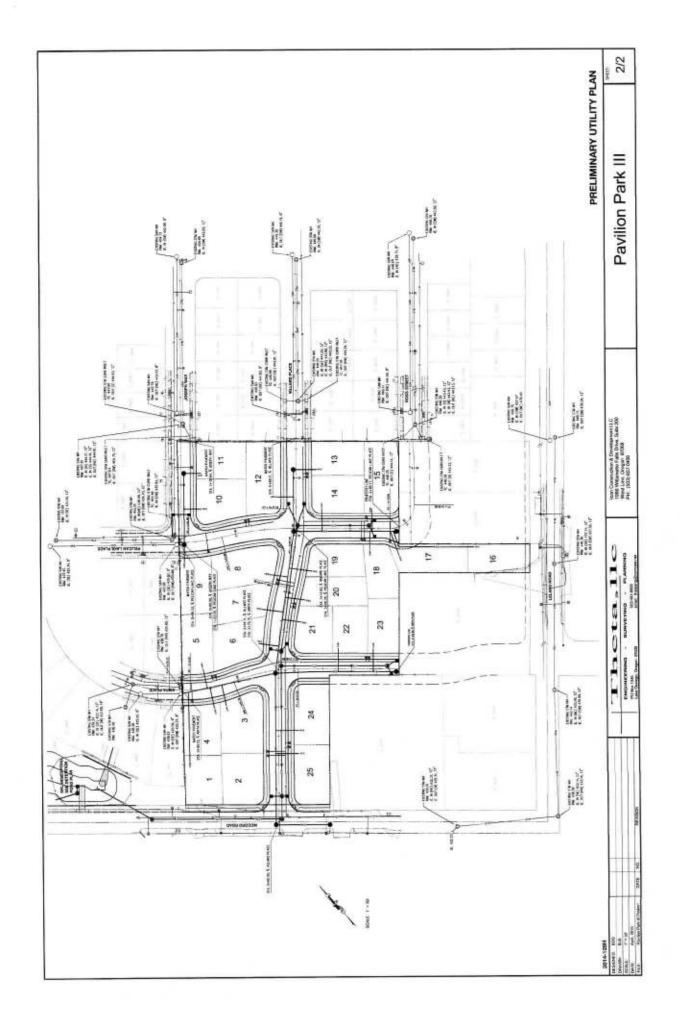
DESIGN HYD:	3.78	2.35	2.34	3.00	9038
TEST HYD 1:	2.74	2.07	.92	2.55	7290
TEST HYD 2:	2.17	.81	81	1.95	5190

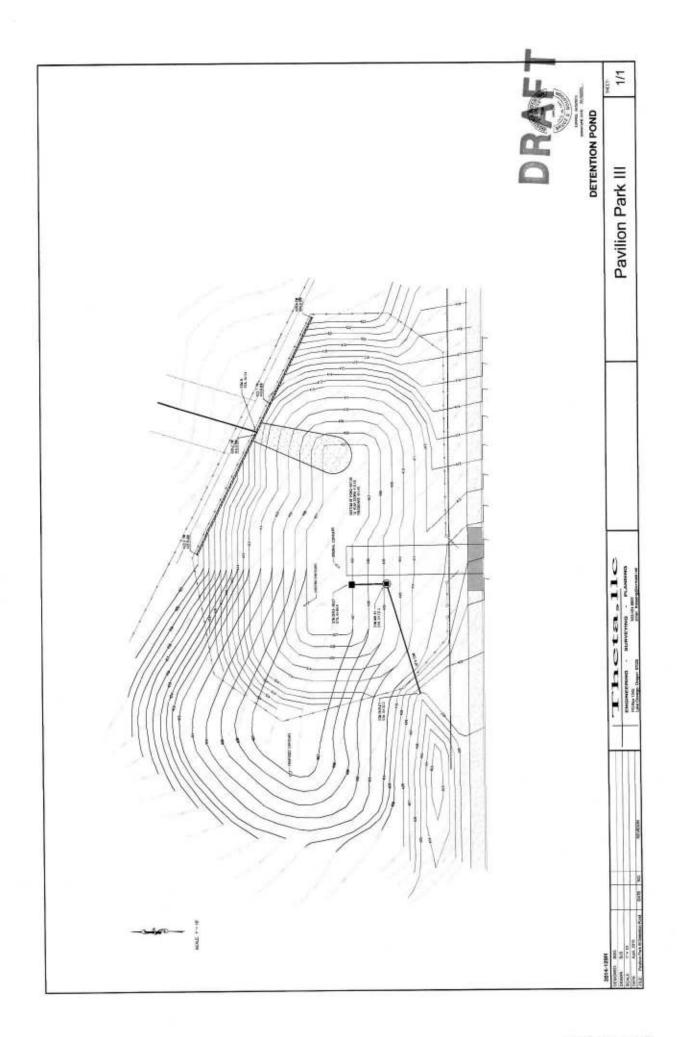
SPECIFY: D - DOCUMENT, R -REVISE, A - ADJUST ORIF, E -ENLARGE, S -STOP

## PRELIMINARY DESIGN:

A proposed addition to the detention and water quality pond is proposed at the existing facility northwesterly of the property. The original calculations were not available but a preliminary grading plan illustrated that enlarging the pond by the required 9038 CF is practical. In the final analysis this new development will be intergraded and adjusting the orifices to reflect both developments. Infiltration and water quality requirements will be met by the type A pond configuration.

# Appendix





#### 4.1.2.1 RAINFALL DISTRIBUTION

The rainfall distribution to be used within the City is the design storm of 24-hour duration based on the standard SCS Type 1A rainfall distribution (See Figure 4-2).

Table 4-1 below links the total depth per year of reoccurrence.

Reoccurrence Year	Total Depth
2	2.6
5	3.1
10	3.4
25	4.0
50	4.4
100	4.5

# Table 4-3 MODIFIED CURVE NUMBERS

SCS Western Washington Runoff Curve Numbers

Runoff curve numbers for selected agricultural, suburban, and urban land use for Type 1A rainfall distribution, 24-hour storm duration. (Published by SCS in 1982)

				1000	
TO A TOMOR LAN	BEST DESCRIPTIONS FOR			ko) (s chich	
7 (2016)	GENERAL AND VERSEA	4 7		(0)	
Cultivated land	Winter Condition	86	91	94	95
Mountain Open Areas:	Low growing brush and grassland.	74	82	89	92
Meadow or pasture:		65	78	85	89
Wood or forest land:	Undisturbed	42	64	76	81
	Established second growth <sup>2</sup>	48	68	78	83
	Young second growth or brush	55	72	81	86
Orchard:	With over crop	81	88	92	94
	, golf courses, cemeteries, landscaping		5520	22.2	
Good Condition:	Grass cover on > =75% of area	68	80	86	90
Fair Condition:	Grass cover on 50-75% of area	77	85	90	92
Gravel Roads and Parking	Lots:	76	85	89	91
Dirt Roads and Parking Lo		72	82	87	89
Impervious surfaces, paver		98	98	98	98
Open water bodies:	Lakes, wetlands, ponds, etc.	100	100	100	100
Single Family Residential	3.				
Dwelling unit/gross acre	% Impervious <sup>4</sup>				
1.0 DU/GA	15				
1.5 DU/GA	20	1			
2.0 DU/GA	25	1			
2.5 DU/GA	30	Select a separate curve			
3.0 DU/GA	34				
3.5 DU/GA	38			rvious a	
4.0 DU/GA	42			rtions o	f the
4.5 DU/GA	46	site or	basin.		
5.0 DU/GA	48				
5.5 DU/GA	50				
6.0 DU/GA	52				
6.5 DU/GA	54				
7.0 DU/GA	56				
Planned Unit Developmen	ts, % impervious4			ate curv	
condominiums, apartment commercial businesses & industrial areas <sup>3</sup>		imper		rvious a ortions o	

Modified by KCFW, 1995.

<sup>3</sup> Assumes roof and driveway runoff is directed into street/storm system.

Print Date: 04/14/00 10:40 AM
Wile Name: HAWRDPILES-BORSTORMMAN/NEW-CHAPALDOC

Chapter 4, Page 12

For a more detailed description of agricultural land use curve numbers, refer to National Engineering Handbook, Sec. 4, Hydrology, Chapter 9, August 1972.

<sup>&</sup>lt;sup>4</sup> The remaining pervious areas (lawn) are considered to be in good condition for these curve numbers.

201	AND 'k' Value Used in Time Calculations for Hydrographs	
00000000	Sheet Flow Equation Manning's Values (for Initial 300 ft. of travel)	T.
	ooth surfaces (concrete, asphalt, gravel, or bare hand packed soil)	0.01
	ow fields or loose soil surface (no residue)	0.05
	tivated soil with residue cover (s # 0.20 ft/ft)	0.06
	tivated soil with residue cover (s > 0.20 ft/ft)	0.17
	ort prairie grass and lawns	0.15
	nse grasses	0.24
	muda grass	0.41
S	nge (natural)	0.13
	ods or forest with light underbrush	0.40
	ods or forest with dense underbrush	0.80
	anning values for sheet flow only, from Overton and Meadows 1976 (See	
	S's TR-55, 1986) "K" Values Used in Travel Time/Time of Concentration	
	ulations Shallow Concentrated Flow (After the initial 300 ft. of sheet	
ilor	v(R = 0.1)	. k.
1.	Forest with heavy ground litter and meadows (n = 0.10)	3
2.	Brushy ground with some trees (n = 0.060)	5
3.	Fallow or minimum tillage cultivation (n=0.040)	8
4.	High grass (n=0.035)	9
5.	Short grass, pasture, and lawns (n=0.030)	11
6.	Nearly bare ground (n=0.025)	13
7.	Paved and gravel areas (n=0.012)	27
94	Channel flow (intermittent) (At beginning of visible channels R#0.2)	k,
1.	Forested swale with heavy ground litter (n=0.10)	5
2.	Forested drainage course/ravine with defined channel bed (n=0.050)	10
3.	Rock-lined waterway (n=0.035)	15
4.	Grassed waterway (n=0.030)	17
5.	Earth-lined waterway (n=0.025)	20
6.	CMP pipe (n=0.024)	21
7.	Concrete pipe (0.012)	42
	Other waterways and pipe 0.508/n	
8.	nnel flow (Continuous stream, R#0.4)	<u>k.</u>
8.	The state of the s	20
8. Cb. 9.	Meandering stream with some pools (n=0.040)	
8. Ch. 9. 10.	Rock-lined stream (n=0.035)	23
8.		

100.0%	6.3		34	Totals for Area of Interest
43.3%	2.7	С	Cottrell silty clay loam, 2 to 8 percent slopes	248
56.7%	3.6	С	Bornstedt silt loam, 0 to 8 percent slopes	88
Percent of AOI	Acres in AOI	Rating	Map unit name	Map unit symbol
9			Summary by Map Unit — Clackamas County Area, Oregon (OR610)	Summary by Map Uni
8			Tables — Hydrologic Soil Group — Summary By Map Unit	īables — Hydrologic S

